

# Flow Assessment of Self-Compacted Concrete incorporating Fly Ash

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**Abstract**—Flow ability is one of the prime characteristics of good concrete and plays a key role in Self Compacting Concrete (SCC). This study assesses flow ability of SCC in which cement content was reduced and replaced by fly ash. Several mixes were prepared by using 5% to 30% fly ash. From the experimental work it was realized that by adjusting a suitable percentage of Super Plasticizer (SP), fly ash dosage can be increased with satisfactory fresh properties of concrete in accordance with the guidelines of EFNARC. At  $T_{50\text{cm}}$  time the optimum amount of SP was 2%. It was noted that with increase in dosage of fly ash the slump flow increased but  $T_{50\text{cm}}$  and V-funnel time were reduced. J-Ring height value varied from 9 to 10 for all the mixes.

**Keywords**—self compacting concrete; flow ability; fly ash; slump flow

## I. INTRODUCTION

Self Compacting Concrete (SCC) is popular because it is a workable concrete with a satisfactory level of strength. It is very useful for narrow sections with ignorable level of segregation and higher flexibility [1, 2]. SCC is also proved to be very useful when reinforcement is very congested. SCC offers ease in pouring compared to ordinary concrete and hence has more commercial benefits [2, 3]. Although compared to ordinary concrete, SCC is a little more expensive but this can be compensated by replacing cement content with supplementary cementitious materials [4-6]. The incorporation of cementitious materials such as metakaolin ash in SCC not only enhances its properties at fresh and hardened state but also decreases the cost of materials and the amount of produced  $\text{CO}_2$  [7-12]. Fly Ash (FA) is one of the commonly used cementitious materials used in producing economical and eco-friendly SCC with a satisfactory level of performance [13, 14]. It is also useful in reducing the necessary quantity of Super Plasticizer (SP) for maintaining the required slump flow [15].

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The overall performance of SCC can be further improved by using mixtures of different fillers of FA with lime stone and/or any other natural pozzolanic materials [16, 17]. The benefits of incorporating cement replacement materials depend on their type, dosage, and physical and chemical composition [18]. The use of FA is very helpful in decreasing the requirement of water in the manufacturing of SCC with a liquefied consistency [19]. On the other hand, when Palm Oil Fuel Ash (POFA) was used instead of Ordinary Portland Concrete (OPC), it increased its initial and final setting time but reduced passing ability. This has gained high importance in concrete technology [21-25]. POFA and FA have been used in [26] in SCC individually and combined. It was observed that FA shows better performance than POFA in all measured characteristics of SCC mixes. Furthermore, it was also reported that the use of higher quantity of POFA and FA has a momentous possibility for medium strength concrete. Authors in [25] used blends of FA, GGBS and SF for assessing SCC and found that FA is the major material in decreasing the compressive strength of the concrete, whereas authors in [21] used FA, marble powder (M) and limestone filler (LF) in SCC and found that the compressive strength of SCC with binary concrete samples was better than of concrete with FS at an early age but there was no difference at a later age.

## II. EXPERIMENTAL PROCEDURE

Figure 1 shows the flowchart of the current research, which starts from literature review, goes to the selection of materials, physical and chemical analysis of materials, and mix design by ENNARC [1], and evaluates the fresh properties of concrete reaching the final conclusions. OPC conforming ASTM-C150M-18 and class F Fly Ash (FLA) as per ASTM C 618-17 were used.

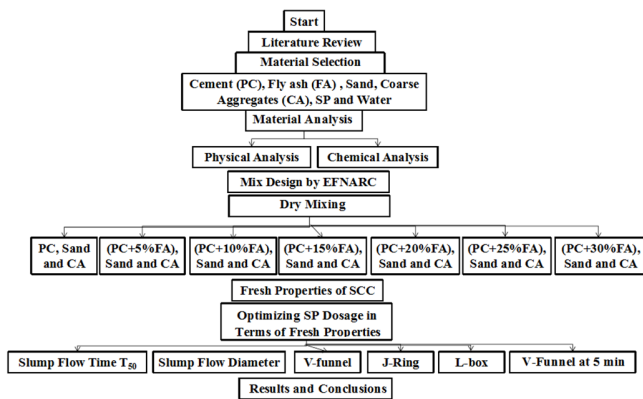


Fig. 1. Flowchart showing the work methodology

Figures 2-3 demonstrate the chemical compositions of OPC and FLA respectively. Table I shows the values of physical and chemical analysis. Fine Aggregates (FA) passed from #4 sieves and Coarse Aggregates (CA) with 13mm maximum size were used throughout the study. The gradation of aggregates presented in Table II. A poly-carboxylate based SP was used to maintain the required workability. In order to investigate the flow characteristics of SCC, seven different mixtures were produced in total with varying percentage replacement of cement by FLA with one being the control mixture as shown in Table III. The fresh properties checked were filling ability, passing ability and segregation resistance. The filling ability was evaluated by slump flow time  $T_{50sec}$ , slump flow diameter, and V-funnel time. The passing ability was measured with J-ring height, and L-box height ratio. V-funnel time at 5 minutes was recorded as a segregation resistance test.

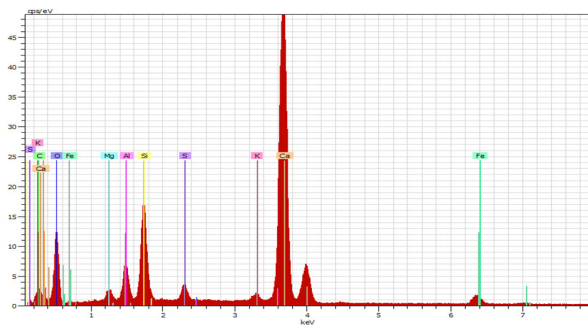


Fig. 2. Chemical composition of OPC

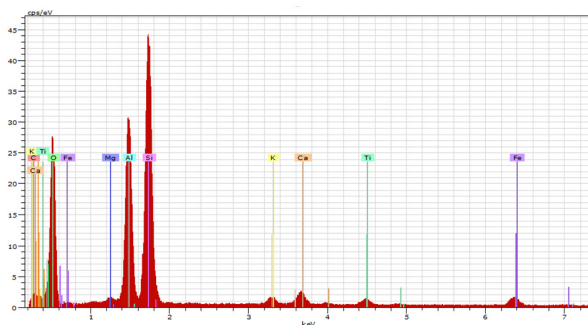


Fig. 3. Chemical composition of FLA

TABLE I. MATERIALS REQUIRED FOR 1m<sup>3</sup> OF SCC

Mix ID	Mix description	OPC kg/m <sup>3</sup>	FLA kg/m <sup>3</sup>	Binder kg/m <sup>3</sup>	FA kg/m <sup>3</sup>	CA kg/m <sup>3</sup>	W/B	Water kg/m <sup>3</sup>	SP kg/m <sup>3</sup>
M1	PC	550	0	550	870	880	0.34	187	11
M2	5% FLA	522.5	27.5	550	870	880	0.34	187	11
M3	10% FLA	495	55	550	870	880	0.34	187	11
M4	15% FLA	467.5	82.5	550	870	880	0.34	187	11
M5	20% FLA	440	110	550	870	880	0.34	187	11
M6	25% FLA	412.5	137.5	550	870	880	0.34	187	8.25
M7	30% FLA	385	165	550	870	880	0.34	187	5.5

TABLE II. OPC AND FLA PHYSICAL AND CHEMICAL PROPERTIES

	OPC (%)	FLA (%)
SiO <sub>2</sub>	15.1	47.98
Al <sub>2</sub> O <sub>3</sub>	6.27	30.4
MgO	2.089	0.945
CaO	59.35	2.994
Fe <sub>2</sub> O <sub>3</sub>	4.003	4.07
K <sub>2</sub> O	1.421	1.84
SO <sub>3</sub>	3.495	-
TiO <sub>2</sub>	-	2.185
Loss of ignition	2.64	1.32
Specific gravity	3.25	2.13

TABLE III. SIEVE ANALYSIS OF FINE AND COARSE AGGREGATES

Sieve size (mm)	Fine aggregates	Coarse aggregates
	Passing (%)	Passing (%)
13.2	100	95.43
9.5	100	46.1
4.75	99.8	0.35
2.36	83.96	0
1.18	66.42	0
0.6	54.1	0
0.3	38.1	0
1.15	9.1	0
pan	0	0

### III. RESULTS AND DISCUSSION

#### A. SP Dosage Optimization

It is well recognized that SP is added in SCC to achieve the required workability. Therefore, it was necessary to acquire the optimized dosage of SP in the selected mixes. For this purpose, slump flow time  $T_{50cm}$  tests were conducted to get the optimized dosage of SP from the trial batches of each mix.  $T_{50cm}$  time should be within the range of 2-6s. The workability of SCC increases with increase in the dosage of SP as can be seen in Table IV. Table V shows the observed results of fresh characteristics during various lab tests.

TABLE IV. VARIATION OF SLUMP FLOW TIME WITH SP DOSAGE

SP dosage	1	1.5	2	2.5	3
Mixes	$T_{50cm}$ slump flow time (sec)				
Control	12	9.3	4.6	--	--
5%	--	--	4.4	3.9	2.6
10%	--	--	4.3	3.3	3
15%	--	--	4.2	4	2.3
20%	--	--	4.1	3.1	2
25%	--	4.2	3.2	2.5	2.1
30%	3.9	3.6	3	2.1	--

**B. Filling Ability**

The filling ability was assessed through ( $T_{50cm}$ ), slump flow diameter and V-funnel tests. Figures 4-6 show the variation of slump flow time, slump flow diameter, and V-funnel flow time respectively, for the tested mixes including control. It can be seen from these figures that  $T_{50cm}$  and V-funnel time decrease with increasing in fly ash percentage. Likewise, slump flow diameter is slightly increasing for increased replacement levels of fly ash due to the viscosity modifying properties of fly ash. The filling ability values of all mixes with the optimised dosages of SP were within the required range [1].

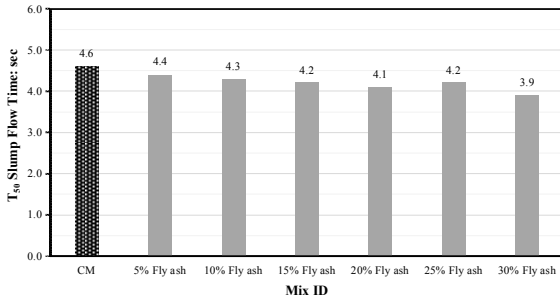


Fig. 4. Variation in  $T_{50cm}$  with fly ash dosage

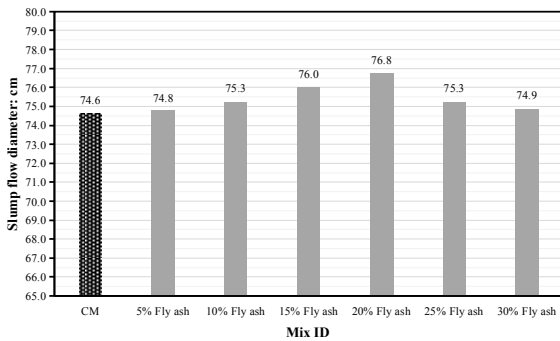


Fig. 5. Variation in slump flow diameter with fly ash dosage

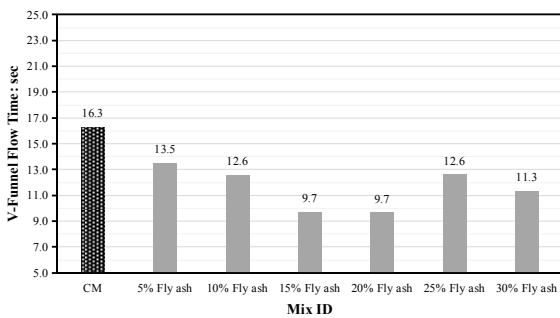


Fig. 6. Variation in V-funnel flow time with fly ash dosage

**C. Passing Ability**

Passing ability is a major characteristic of SCC in maintaining concrete flow. When structural components are very narrow or highly reinforced with small spaces between them, passing ability is more important than the uniformity of concrete. At laboratory level, this can be performed through the

J-Ring and L-box tests. The height of J-Ring and L-box height ratio ( $H2/H1$ ) are given in Table V which depicts the passing ability of prepared mixes confirming the desired level as suggested by EFNARC.

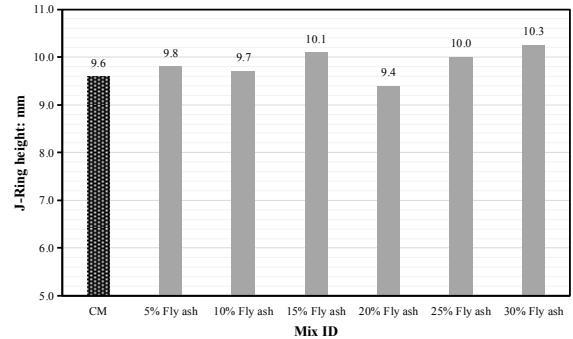


Fig. 7. Variations in J-Ring height with fly ash dosage

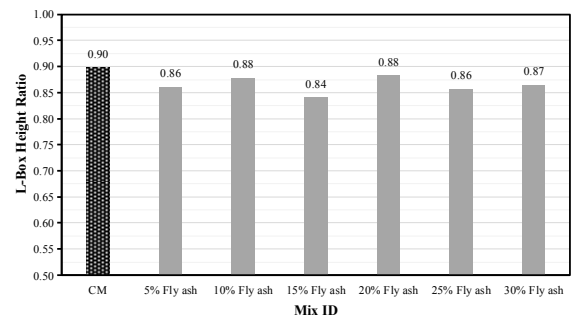


Fig. 8. Variations in L-Box height ratio with fly ash dosage

**D. Segregation Resistance**

The consistency of the mixes was inspected through the V-Funnel test at 5min. It is observed from Figure 9 that for all the mixes the V-funnel flow time was in the required range (8-25s) according to the EFNARC guidelines.

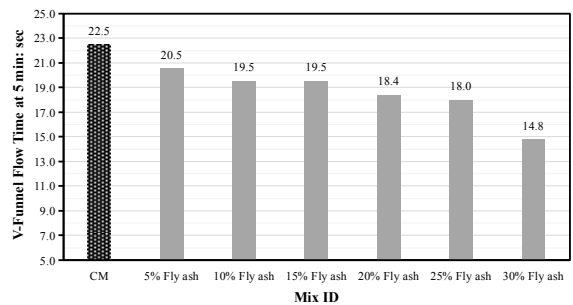


Fig. 9. Variation in V-Funnel flow time (at 5min) with fly ash dosage

**IV. CONCLUSIONS**

The fresh properties assessed by the workability tests of SCC depend upon the dosage of fly ash in the mix which can be adjusted by a suitable percentage of SP.  $T_{50cm}$  is the key test for the fresh properties of SCC, while  $T_{50cm}$  time between 4 and 5s is considered optimum. On the basis of  $T_{50cm}$  time, the optimized dosage of SP for mixes are (CM 2%), (5FA 2%),

(10FA 2%), (15FA 2%), (20FA 2%), (25FA 1.5%) and (30FA 1%). Slump flow diameter increases and  $T_{50cm}$  and V-funnel time decrease with increase in the percentage of fly ash. J-ring height for all the mixes ranged from 9 to 10mm. The values of

L-box height ratio for all mixes are between 0.8 and 1.0. It was observed that filling ability, passing ability and segregation resistance for all the mixes with the optimised SP dosage were within the required range as per EFNARC.

TABLE V. SCC FRESH PROPERTIES

Mix	Filling ability			Passing ability		Segregation resistance
	Slump flow time (s) limits [2-5] sec	Slump flow diameter (cm) limits [65-80] cm	V-funnel time (s) limits [6-25] s	J- ing height (mm) limits [0-10] mm	L-box height ratio limits [0.8-1]	V-funnel time at 5min (s) limits [8-25] s
PC	4.6	74.6	16.3	9.6	0.90	22.5
5%	4.4	74.8	13.5	9.8	0.86	20.5
10%	4.3	75.3	12.6	9.7	0.88	19.5
15%	4.2	76.0	9.7	10.1	0.84	19.5
20%	4.1	76.8	9.7	9.40	0.88	18.4
25%	4.2	75.3	12.6	10.0	0.86	18.0
30%	3.9	74.9	11.3	10.3	0.87	14.8

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